

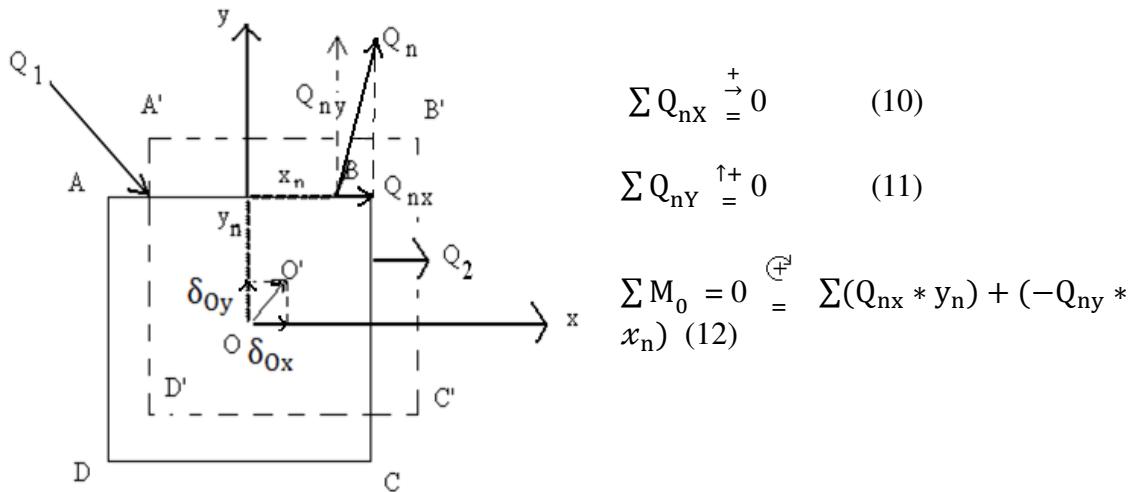
DEFLECTION BY VIRTUAL WORK METHOD

(A) Principle of virtual work for Rigid Bodies.

Consider a truly rigid body which is in static equilibrium under a system of forces Q_i .

Suppose this rigid body is TRANSLATED a small amount (say δ_0) by some other cause (which is independent of Q_i force system).

Since δ_0 is small all Q_i forces are assumed to maintain the same position and direction relative to the rigid body and to each other, and hence to maintain EQUILIBRIUM during this rigid translation:

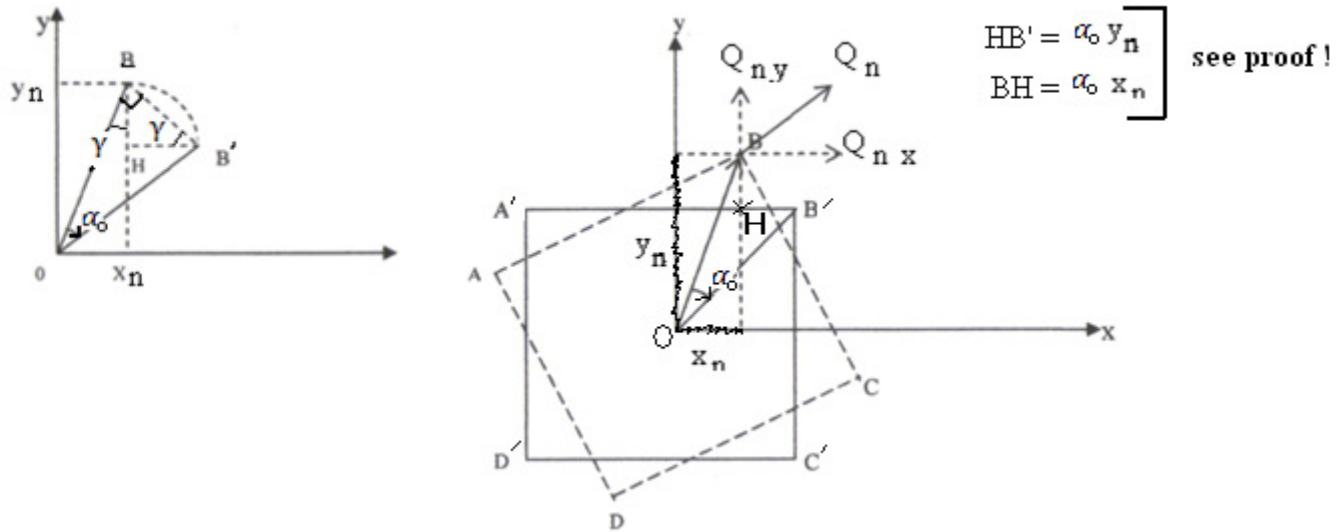


Work done by the

$$Q_i \text{ forces} = W_Q = \sum (Q_{nx} \delta_{0x} + Q_{ny} \delta_{0y}) = (\delta_{0x}) \sum Q_{nx} + (\delta_{0y}) \sum Q_{ny} \quad (13)$$

$$W_Q = 0 \quad (14)$$

Similarly, if the rigid body is ROTATED a small amount α_0 by some other cause (which is independent of Q_i forces), then:



Since: α_0 is a small angle $\Rightarrow \widehat{OBB'} \approx 90^\circ$

Hence $\widehat{OBH} \equiv \gamma \cong \widehat{HB'B}$

We have:

$$OB = \sqrt{x_n^2 + y_n^2} \quad \text{And} \quad BB' \approx (OB)(\alpha_0^{\text{rad}}) = \sqrt{x_n^2 + y_n^2} (\alpha_0)$$

$$HB' = BB' * \cos(\gamma) = BB' * \frac{y_n}{OB} = \sqrt{x_n^2 + y_n^2} (\alpha_0) * \frac{y_n}{\sqrt{x_n^2 + y_n^2}} = \alpha_0 y_n$$

$$BH = BB' * \sin(\alpha) = BB' * \frac{x_n}{OB} = \sqrt{x_n^2 + y_n^2} (\alpha_0) * \frac{x_n}{\sqrt{x_n^2 + y_n^2}} = \alpha_0 x_n$$

Now,

$$W_Q = \sum(Q_{nx} * HB' - Q_{ny} * BH) = \sum(Q_{nx} * \alpha_0 y_n - Q_{ny} * \alpha_0 x_n) \dots \dots (15)$$

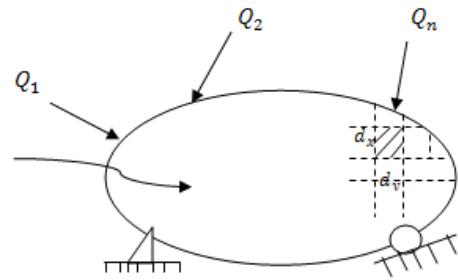
$$W_Q = (\alpha_0) \underbrace{\sum(Q_{nx} y_n - Q_{ny} x_n)}_{\text{Zero (see Eq. 12)}} = 0$$

\

(B) Principal of Virtual Work for Deformable Bodies:

Step 1:

→ This deformable body is in Equilibrium under the applied virtual forces Q_i and support reactions.



Step 2:

Now, suppose the body is subjected to a small change in shape, caused by another load P system (which is independent from O_i system)

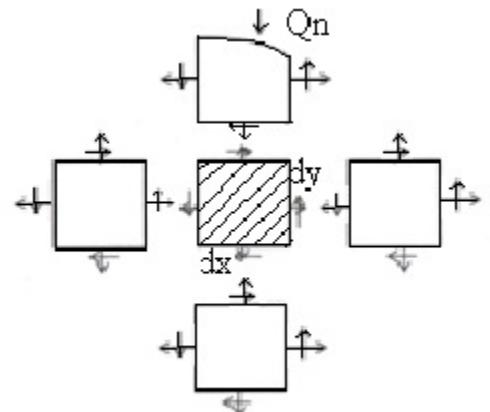
Step 3:

Any INTERIOR particle, with dimensions d_x and d_y , will be displaced in its boundary and will have its virtual work to be canceled by its adjacent particle(s).

Step 4:

Virtual work done on the EXTERIOR (or boundary)

Particle is given by:



Where:

dw_B = due to RIGID body Translation & *Rotation* = 0 (see Eq. 14 & Eq. 15)

dw_d = due to deformable body

Hence:

Hence: $dw_s = dw_d \Rightarrow \int dw_s = \int dw_d \quad \text{or} \quad [w_s = w_d] \dots \dots \dots (17)$

(C) How to Evaluate W_s & W_d in Eq. (17) ?

In Eq. (17), one has: $W_s = W_d$

Where W_s & W_d represents the External Virtual Work (VW),

and the Internal V.W., respectively.

The internal V.W. ($=W_d$) of deformation can be computed as:

With:

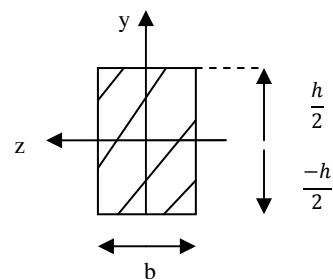
$$(W_d)_{\text{Axial}} = (F_Q) * \left(\Delta L_{\text{real}} = \frac{F_p L}{AE} + \alpha_T * \Delta T * L + \Delta L_{\text{Fabrication Error}} \right) \dots (19a)$$

$$(W_d)_{\text{Bending}} = \sum_{\text{All members}} \int_{s=0}^L \int_{y=-\frac{h}{2}}^{\frac{h}{2}} \left(\frac{M_Q y}{I} \right) (bdy) * \left(\frac{M_p y}{IE} \right) * (ds) = \sum \int_0^L M_Q * \frac{M_p ds}{EI} \dots (19b)$$


 The diagram illustrates the decomposition of the virtual force term. It shows a horizontal line representing the beam with arrows indicating the length. Above the line, four double-headed arrows represent the components: 'Stress' (between the top and bottom boundaries), 'Area' (between the boundaries and the neutral axis), 'Strain' (between the neutral axis and the bottom boundary), and 'Length' (between the two boundaries). Below the line, a double-headed arrow labeled 'Virtual Force' is shown, indicating the overall force is the product of these four components.

Since:

$$\int_{y=-\frac{h}{2}}^{\frac{h}{2}} y^2 (b d y) = \left[\frac{b y^3}{3} \right]_{-\frac{h}{2}}^{\frac{h}{2}} \equiv I$$



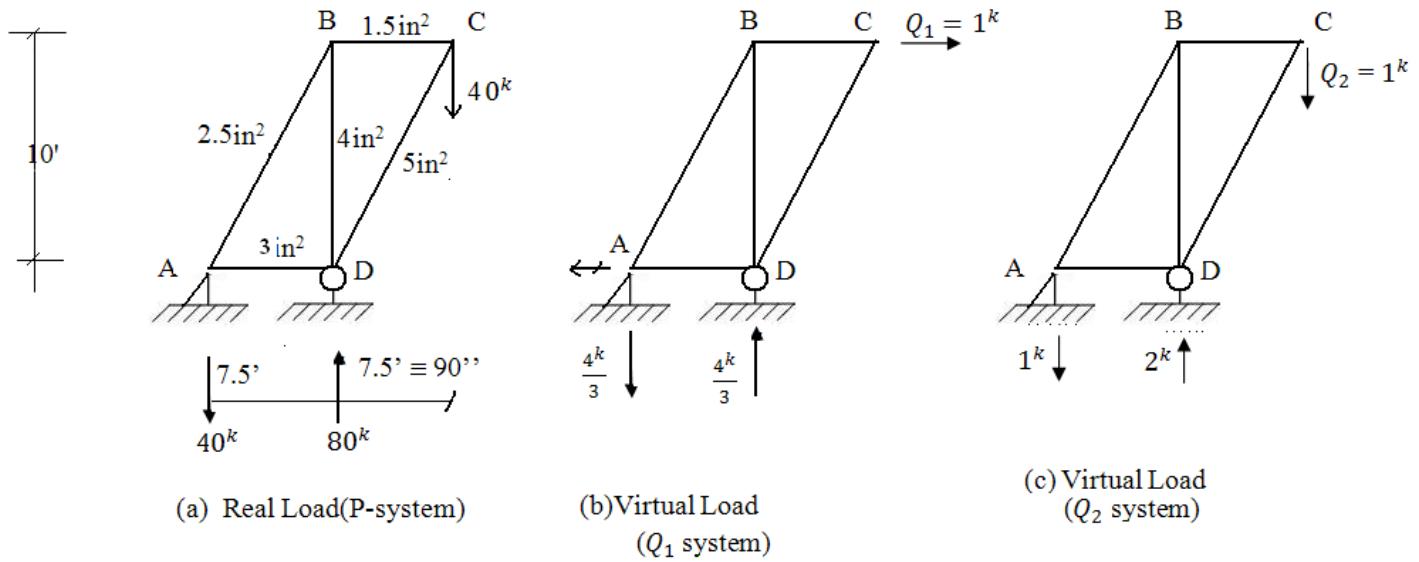
$$(W_d)_{\text{Torsion}} = (T_Q = \text{Virtual Torque}) * \left(\frac{T_{PL}}{JG} \right) = \text{Real angle of twist}. \dots \dots \quad (19c)$$

Where k =a constant, depending on the cross-sectional shape of the beam

Important Notes

- The nature of the applied “VIRTUAL” Load or Moment does depend on Translation or Rotation that we want to compute (as will be demonstrated through many V.W. examples).
- In Eqs. (19a - 19d), the subscripts Q and P represents “Virtual” and “Real” loads, respectively.

Example 1: 2-D Truss with Applied Load



Units used = Kips, Inches

E = Young Modulus (K/in²)

For the truss (shown in Fig. a), find the Horizontal & Vertical displacements at Joint C.

Solution for Example 1

- For finding the Horizontal displ. at Joint C ($\Delta c_{\text{horiz.}}$); one applies Virtual Load Q_1 (Fig. b)
-
- For finding the Vertical displ. at Joint C ($\Delta c_{\text{Vert.}}$); one applies Virtual Load Q_2 (Fig. c)
- From V.W Formulas (see Eqs. 17; 19 a → 19d), one has ;

$$W_s + W_{\text{settlement}} = W_d \iff (Q_1 = 1^k \rightarrow (\Delta c_{\text{horiz}} ??) + W_{\text{settlement}} = \sum_{\text{All members}} F_{Q_i} \left(\frac{F_p L}{AE} \right)_i \dots (20)$$

$$(Q_2 = 1^k \downarrow) (\Delta c_{\text{Vert.}} ??) + W_{\text{settle}} = \sum_i F_{Q_{2i}} \left(\frac{F_p L}{AE} \right)_i \dots \dots \dots \quad (21)$$

- From Static equilibrium , and applying Methods of Joints in figures (a + b + c), one gets:

Member	Length \equiv L	Area \equiv A	$\frac{L}{A}$	F_p	F_{Q_1}	F_{Q_2}	$F_{Q_1} \frac{F_p L}{A}$	$F_{Q_2} \frac{F_p L}{A}$
AB	150	2.5	60	50	5/3	5/4	5,000	3,750
BC	90	1.5	60	30	1	3/4	1/800	1,350
CD	150	5.0	30	-50	0	-5/4	0	1,875
BD	120	4.0	30	-40	-4/3	-1	1,600	1,200
AD	90	3.0	30	-30	0	-3/4	0	675
							$\sum_1 = 8,400$	$\sum_2 = 8,850$

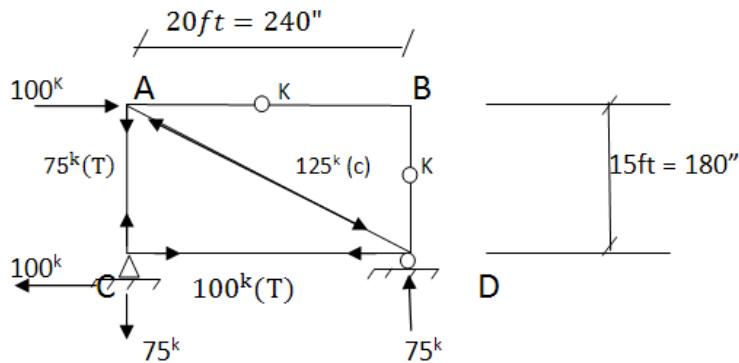
- Hence: $\Delta c_{horiz.} = \frac{+8400}{E}$ and $\Delta c_{Vert.} = \frac{8850}{E}$

Example 2: 2-D Truss with Applied Load

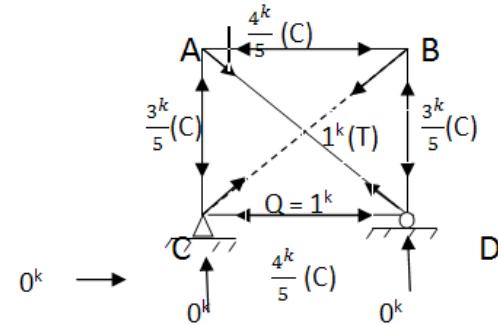
Find the “relative displacement” of joints C and B.

Area of each bar = $10 \text{ in}^2 = A$

Young Modulus = $30,000 \frac{\text{k}}{\text{in}^2} = E$



(a) Real Load (P-System)



(b) Virtual Load (Q-system)

$$\begin{aligned}
 W_s + W_{\text{settlement}} &= \left(Q = 1^k \right) (\Delta_{CB}) + 0 = \sum_i F_{Qi} \left(\frac{F_p L}{AE} \right)_i \\
 &= \sum \left\{ \left(-\frac{3}{5} * 75^k * 180'' \right) + \left(1^k * -125^k * 300'' \right) + \left(-\frac{4}{5} * 100^k * 240'' \right) \right\} * \left(\frac{1}{AE} \right) \\
 &= -\frac{5400}{AE}
 \end{aligned}$$

$$\text{Thus: } \Delta_{CB} = \frac{-5400}{AE} \quad (\text{or} \quad \Delta_{CB} = \frac{5400}{AE})$$

Example 3: Plane Truss
(with Settlements, Fabrication Errors)
The truss shown is subjected to support settlements

At joints A $\{3'' \rightarrow, 2'' \uparrow\}$ and B $\{2'' \rightarrow, 3'' \downarrow\}$

What should be the length of member AD in order to have $(\Delta_D)_{\text{Horizontal}} = 0''$?

Given cross-sectional area of each bar = $20 \text{ in}^2 = A$

$$\text{Young Modulus} = 30,000 \frac{\text{K}}{\text{in}^2} = E$$

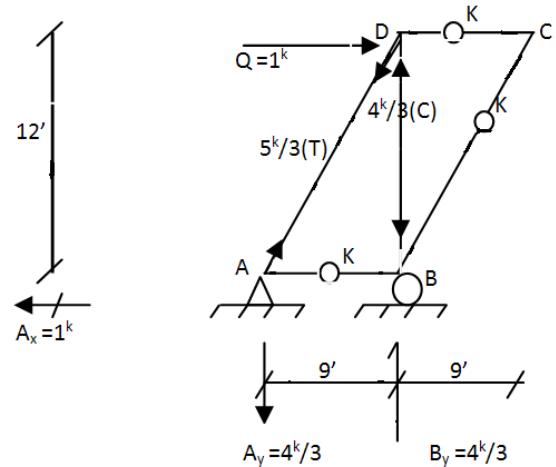


Fig. 1 : Virtual Load Applied

Solution

- Due to settlements' effects, horizontal displ. at joint D can be computed as:

$$W_s + W_{\text{settlement}} = \cancel{W_d} \rightarrow \text{zero}$$

$$(Q = 1^k \rightarrow) (\Delta_{D_{\text{Horiz.}}}^1) + (+A_x = +1^k)(+3'') + (-A_y = -\frac{4^k}{3})(2'') + (B_y = \frac{4^k}{3})(-3'') = 0$$

Thus: $(\Delta_{D_{\text{Horiz.}}}^1) = +(\frac{11}{3}) \text{ in}$, or joint D moves to the right.

- In order to prevent the Horiz. displ. at joint D (due to settlement), length of member AD has to be adjusted, so that joint D will move back to the left, by the same amount.

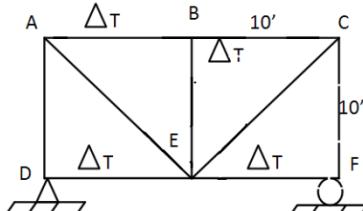
$$(W_s = 1^k * \frac{-11 \text{ in}}{3}) + W_{\cancel{\text{settle}}} = \left(F_{Q_{AD}} = \frac{5^k}{3} \right) \left(\Delta L_{\text{Fabrication}} ?? \right) \rightarrow (\Delta L_{\text{Fab.Err}})_{AD} = -\frac{11''}{5}$$

0

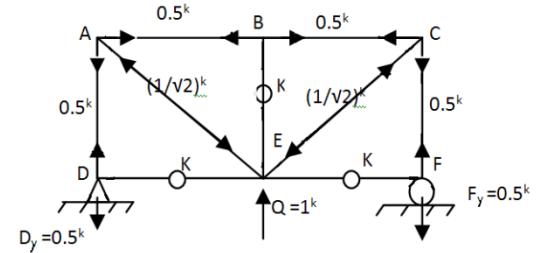
Example 4: Plane Truss with Temperature, Pre-strain (or Fabrication Error), Support settlements ... effects

Given: $E = 30,000 \frac{k}{in^2}$
 $\alpha = 6.5 * 10^{-6} \frac{in}{in^{\circ}F}$

Find: Vertical displ. at joint E.



(a) Real "load" P-System



(b) Virtual Load Q-System

(A) Due to temperature of bars AB, BC, DE & EF is raised by $100^{\circ}F$

$$W_s + W_{\text{settlement}} = W_d$$

$$(Q = 1^k \uparrow)(\Delta_{E_{\text{vert.}}}) + 0 = \sum_i F_{Q_i} (\alpha \Delta T * L)_i \text{ where } \begin{cases} i = \text{bars AB, BC, DE, EF} \\ (F_Q)_{AB} = 0.5^k = (F_Q)_{BC} \\ (F_Q)_{DE} = 0^k = (F_Q)_{EF} \end{cases}$$

$$\text{So: } \Delta_{E_{\text{vert.}}} = +6.5 * 10^{-3} \text{ ft (or moves upward)}$$

(B) Due to Pre-strain (or Fabrication Error)

Bars AB, BC, DE, EF, AD, BE, CF are 2" too short.

Bars AE, CE are $\sqrt{2} = 1.414$ " too short

$$W_s + W_{\text{settle}} = W_d$$

$$(Q = 1^k \uparrow)(\Delta_{E_{\text{vert.}}}) + 0 = \sum (F_{Q_i})(\Delta L_{P_i}) \text{ where } i = \text{AB, ..., CF; AE, CE} \\ = \underbrace{(0.5^k)(-2")}_{\text{AB}} + \dots + \underbrace{(0.5^k)(-2")}_{\text{CF}} + \underbrace{\left(\frac{-1^k}{\sqrt{2}}\right)(-\sqrt{2}^")}_{\text{AE}} + \underbrace{\left(\frac{-1^k}{\sqrt{2}}\right)(-\sqrt{2}^")}_{\text{CE}}$$

$$\text{So: } \Delta_{E_{\text{vert.}}} = -2" \text{ (or moves downward)}$$

(C) Due to settlements at Joint D $\left\{ \begin{matrix} 2'' \leftarrow \\ 1'' \downarrow \end{matrix} \right. , \text{ and at Joint F} \left\{ \begin{matrix} 3'' \rightarrow \\ 4'' \downarrow \end{matrix} \right.$

$$W_s + W_{\text{settle}} = W_d$$

$$(Q = 1^k \uparrow)(\Delta_E)_{\text{vert}} + (D_y = -0.5^k)(-1'') + (F_y = -0.5^k)(-4'') = 0$$

So: $\Delta_{E_{\text{vert}}} = -2.5''$ (or moves downward)

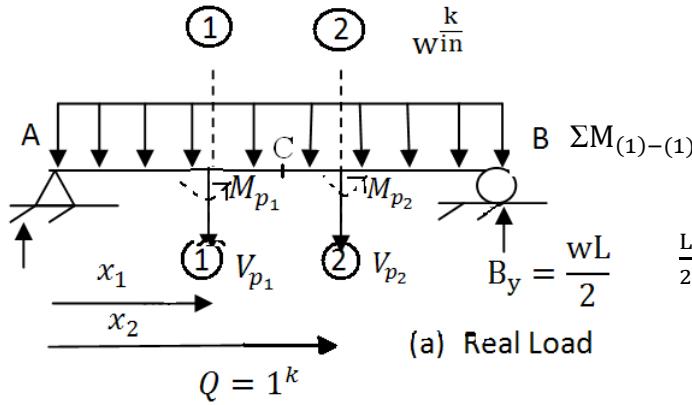
(D) Due to all, combined effects

$$(\Delta_E)_{\text{vert}} = + \left(6.5 * 10^{-3} \text{ft} * \frac{12''}{\text{ft}} \right) + (-2'') + (-2.5'') = -4.42''$$

Example 5: V.W on Beam

Find the vertical deflection Δ_c at mid-point of the beam?

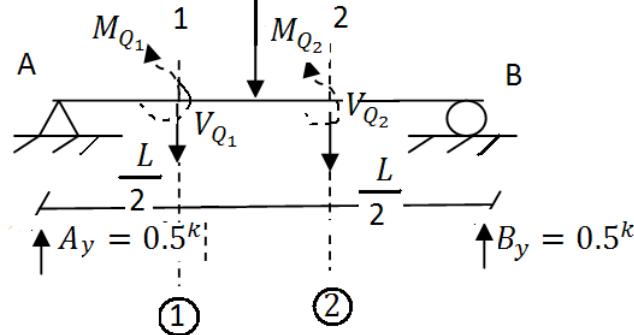
- Use left-side of section (1)-(1) as free



- Similarly:

$$Mp_2 = \frac{wLx_2}{2} - \frac{wx_2^2}{2}$$

for $\frac{L}{2} \leq x_2 \leq L$



(b) Virtual Load

- Use left-side of section (1)-(1) as FBD:

$$\sum M_{(1)-(1)} = 0 = M_{Q_1} - (Ay = 0.5^k)(x_1)$$

$$\text{So: } M_{Q_1} = 0.5x_1 \text{ for } 0 \leq x_1 \leq \frac{L}{2}$$

- Use left-side of section (2)-(2) as FBD:

$$\sum M_{(2)-(2)} = 0 = M_{Q_2} - (Ay = 0.5^k)(x_2) + (1^k)\left(x_2 - \frac{L}{2}\right)$$

$$\text{So: } M_{Q_2} = -0.5x_2 + \frac{L}{2} \text{ For } \frac{L}{2} \leq x_2 \leq L$$

Now, applying V.W. formula (see Eq. 19 b)

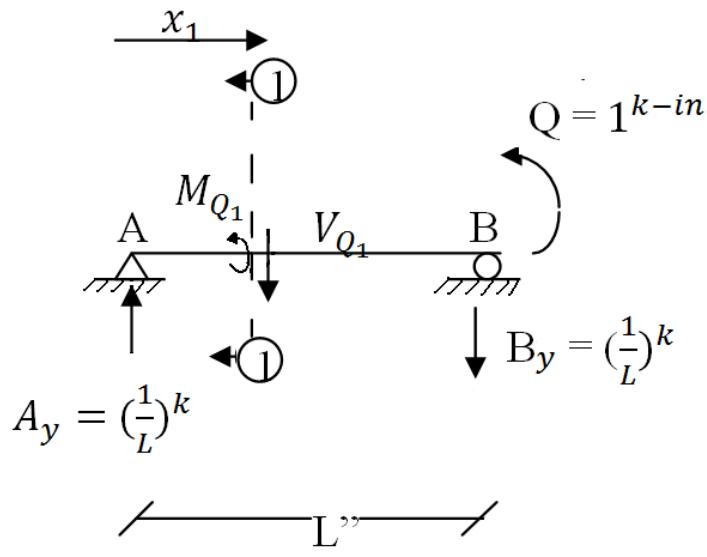
$$W_S + W_{\text{settle}} = \int M_Q * \frac{M_p}{EI} dx$$

$$(Q = 1^k \downarrow)(\Delta_c ??) + 0 = \int_{x_1=0}^{\frac{L}{2}} M_{Q_1} * \frac{M_{P_1}}{EI} dx_1 + \int_{x_2=\frac{L}{2}}^L M_{Q_2} * \frac{M_{P_2}}{EI} dx_2$$

$$\text{Or: } \Delta_c = \int_{x_1=0}^{L/2} (0.5x_1) * \frac{\left(\frac{wLx_1}{2} - \frac{wx_1^2}{2}\right)}{EI} dx_1 + \int_{x_2=\frac{L}{2}}^L \left(-0.5x_2 + \frac{L}{2}\right) * \frac{\left(\frac{wLx_2}{2} - \frac{wx_2^2}{2}\right)}{EI} dx_2$$

$$\text{Hence: } \Delta_c = \frac{+5wL^4}{384EI} \text{ (or point C will move } \underline{\text{downward}}, \text{ as we have assumed for direction of Q)}$$

Example 6: More on V.W. for Beam



For the same beam (Shown in Example 5), find the rotation (θ_B) at Joint B

$$\sum M_{(1)-(1)} = 0 \quad \text{at } B = M_{Q_1} - (Ay = \frac{1}{L})^k \text{kip}(x_1)$$

$$\text{Hence: } M_{Q_1} = \frac{x_1}{L} \text{ for } 0 \leq x_1 \leq L$$

Applying V.W. formula (See Eq. 19b):

$$W_s + W_{\text{settle.}} = W_d$$

Virtual Load (Q-System)

$$(Q = 1^{k\text{-in}} \curvearrowright)(\theta_B) + 0 = \int M_Q \frac{M_p}{EI} dx$$

Or $\theta_B = \int_{x_1=0}^L \left(\frac{x_1}{L} \right) * \frac{\left(\frac{wLx_1}{2} - \frac{wx_1^2}{2} \right)}{EI} dx_1$

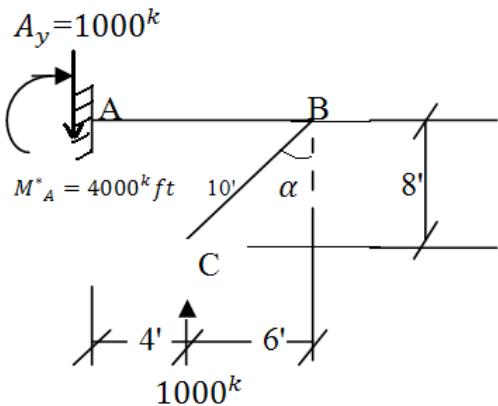
Note:

The M_{p_1} computed earlier (see Example 5) can also be used for entire beam (or $0 \leq x_1 \leq L$)

Example 7: V.W. on Frame

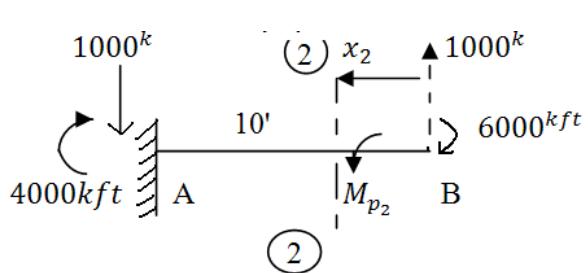
Given: $E = 30,000 \text{ k/in}^2$; $I = 288 \text{ in}^4$

Find: vertical deflection at point C? Rotation at point C?



(a) Real Load, P-System

(b) Free body Diagrams (for P-system)

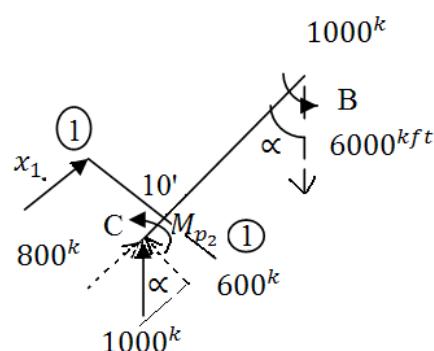


$$\sum M_{(2)-(2)} = 0 \stackrel{\text{clockwise}}{=} M_{p_2} + (1000^k)(x_2) - 600 \text{ kft}$$

So:

$$M_{p_2} = 6000 \text{ kft} - 1000x_2$$

$$0' \leq x_2 \leq 10'$$



$$\sum M_{(1)-(1)} = 0 \stackrel{\text{clockwise}}{=} M_{p_1} - (600^k)(x_1)$$

So:

$$M_{p_1} = 600x_1$$

$$0' \leq x_1 \leq 10'$$

Note: To find vertical deflection at point C, we could have also applied a “Virtual” vertical load $Q_1 = 1000^k$ at point C.

Hence, Fig. (b) can also be used for Q_1 -system !

To find vertical deflection (Δ_c) at joint C

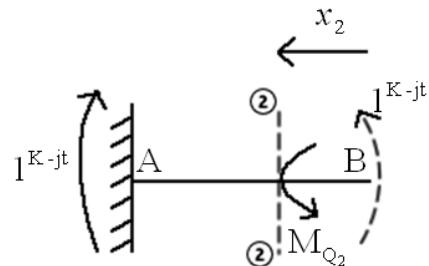
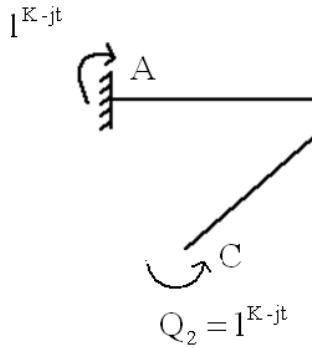
$$W_s + W_{\text{settle}} = W_d$$

$$(Q_1 = 1000^k)(\Delta_c^{\text{ft}}) + 0^{\text{kft}} = \int M_Q * \frac{M_p dx}{EI}$$

$$\begin{aligned} \text{Or } (1000) \Delta_c^{\text{ft}} &= \int_{x_1=0}^{10} M_{Q_1} * \frac{M_{p_1}}{EI} dx_1 + \int_{x_2=0}^{10} M_{Q_2} * \frac{M_{p_2}}{EI} dx_2 \\ &= \int_0^{10} \frac{(M_{p_1})^2}{EI} dx_1 + \int_0^{10} \frac{(M_{p_2})^2}{EI} dx_2 \end{aligned}$$

$$(1000) \Delta_c^{\text{ft}} = \int_0^{10} \frac{(600x_1)^2}{EI} dx_1 + \int_0^{10} \frac{(6000^{\text{kft}} - 1000x_2)^2}{EI} dx_2$$

$$\text{Thus: } \boxed{\Delta_c^{\text{ft}} = \left(\frac{1}{1000}\right) \left\{ \int_0^{10} \frac{(600x_1)^2}{EI} dx_1 + \int_0^{10} \frac{(6000^{\text{kft}} - 1000x_2)^2}{EI} dx_2 \right\}}$$

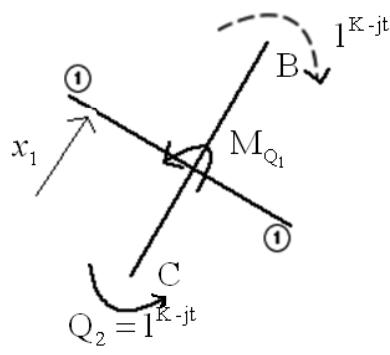


$$\sum M_{(2)-(2)} = 0 \stackrel{\leftarrow}{=} = M_{Q_2} + 1^K\text{-ft}$$

So:

$$M_{Q_2} = -1^K\text{ft}$$

Valid for $0' \leq x_2 \leq 10'$



$$\sum M_{(1)-(1)} = 0 \stackrel{\leftarrow}{=} = M_{Q_1} + 1^K\text{ft}$$

So:

$$M_{Q_1} = -1^K\text{ft}$$

Valid for $0' \leq x_1 \leq 10'$

(C)- Virtual Load (Q_2 -System) and FBD of members AB, BC.

To find ROTATION (θ_c) at joint C

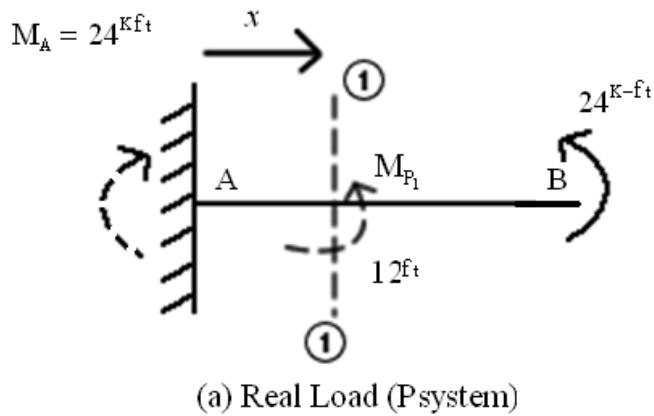
$$W_s + W_{\text{settle}} = W_d$$

$$\left(Q_2 = 1 \text{ Kft} \rightsquigarrow \right) (\theta_c) + 0 \text{ Kft} = \int M_Q * \frac{M_p dx}{EI}$$

$$\text{Or: } \theta_c = \int_{x_1=0}^{10'} M_{Q_1} * \frac{M_{P_1} dx_1}{EI} + \int_{x_2=0}^{10'} M_{Q_2} * \frac{M_{P_2} dx_2}{EI}$$

$$\text{Or: } \boxed{\theta_c = \int_0^{10} (-1 \text{ Kft}) * \frac{(600x_1)}{EI} dx_1 + \int_0^{10} (-1 \text{ Kft}) * \frac{(6000 \text{ Kft} - 1000x_2)}{EI} dx_2}$$

Example 8: V.W. for a Determinate Beam
Find the vertical deflection of joint B.

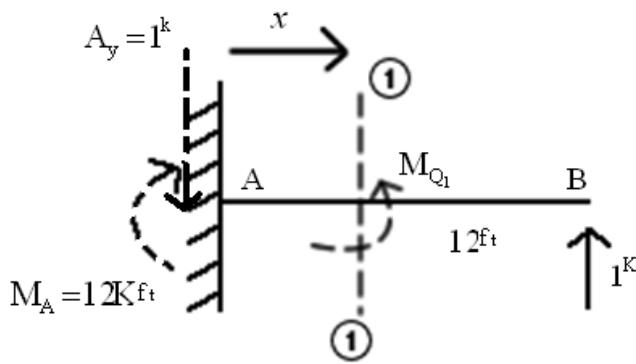


$$W_s = W_d$$

$$(1^k)(\delta_{BV}) + (W_{\text{settle}} = 0) = \int \frac{M_{Q_1} M_{P_1} dx}{EI} \quad (8.1)$$

Use left side of cut section 1)-1) as FBD, then:

$$\sum M_1 = 0 = M_{P_1} - 24^{\text{kft}} \Rightarrow M_{P_1} = 24^{\text{kft}} \quad (8.2)$$



Use left side of the cut section 1)-1) as FBD, then:

$$\sum M_1 = 0 = M_{Q_1} + (1^k)(x) - 12^{\text{kft}}$$

$$\text{Hence: } M_{Q_1} = 12 - x \quad (8.3)$$

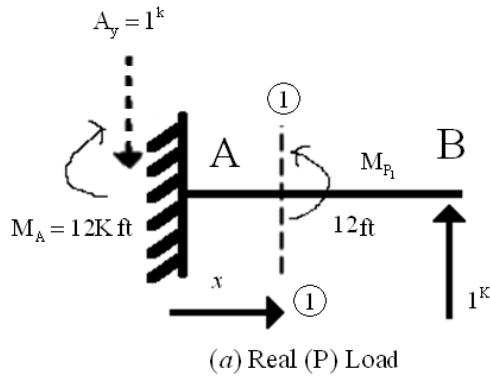
Substitute Eqs. (8.2, 8.3) into Eq. (8.1), one gets

$$\delta_{BV} = \int \frac{(12-x)(24)(dx)}{EI} = \dots$$

(b)- Virtual Load (Q-System)

Example 9: V.W. for Determinate Beam

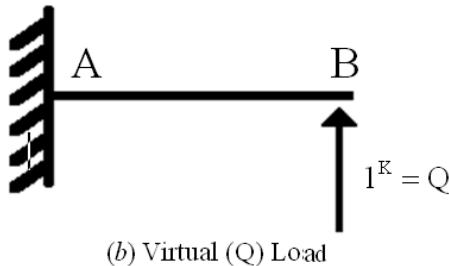
Find the vertical deflection at joint B? (Refer to example 8)



$$W_s = W_d$$

$$(Q = 1^k \uparrow)(\delta_{B_V})$$

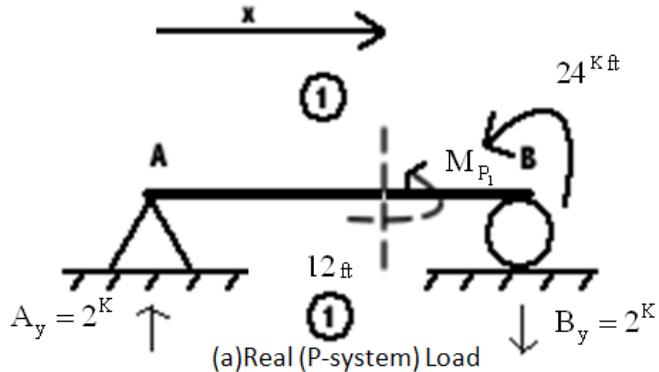
$$+ (W_{\text{settle}} = 0) \int \frac{(M_{q_1}(M_{P_1} = M_{q_1}))}{EI} dx$$



$$\delta_{B_V} = \int \frac{(M_{q_1})^2 dx}{EI} = \int_0^{12} \frac{(12-x)^2}{EI} dx = \dots$$

Example 10: V.W. for simply support, Determinate Beam
Find the ROTATION at joint A.

Use left side of cut section 1)-1) as FBD;



$$\sum M_{(1)} = 0 \quad \text{at } A \quad = M_{P_1} - (2^k)(x) \Rightarrow M_{P_1} = 2x \quad (10.1)$$

$$\sum M_{(1)} = 0 \quad \text{at } B \quad = M_{Q_1} + 1^{\text{kft}} - \left(\frac{1}{12}\right)(x)$$

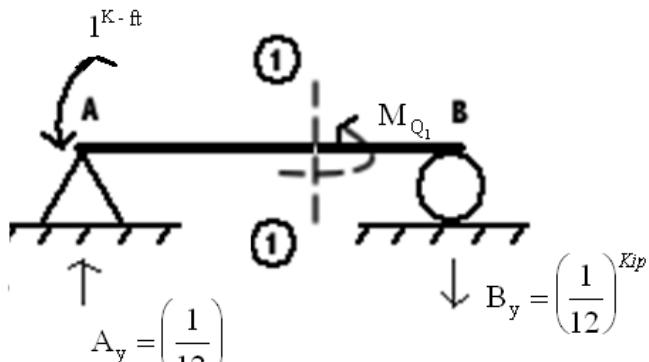
$$\text{Hence: } M_{Q_1} = \frac{x}{12} - 1 \quad \dots \quad (10.2)$$

Use V.W:

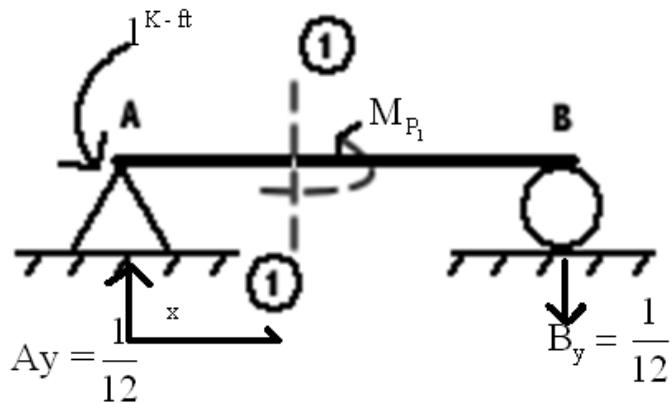
$$W_s + W_{\text{settle}} = W_d$$

$$(Q = 1^{\text{kft}}) \sum (\theta_A) + 0 = \int \frac{M_{Q_1} M_{P_1}}{EI} dx$$

$$\theta_A = \int_0^{12} \left(\frac{x}{12} - 1\right)(2x) \frac{dx}{EI} = \dots$$



Example 11: V.W. for Simply Support, determine beam.
Find the rotation at Joint A.



Use left side of cut section (1)-(1)
as FBD;

$$\begin{aligned}\sum M_1 &= 0 \quad \text{at } \textcircled{1} \\ &= M_{P_1} + 1^{\text{kft}} \\ &\quad - \left(\frac{1}{12}\right)(x)\end{aligned}$$

$$\text{Hence: } M_{P_1} = \frac{x}{12} - 1 \equiv M_{Q_1}$$

Real (P-system) \equiv Virtual (Q-system)

Apply VW:

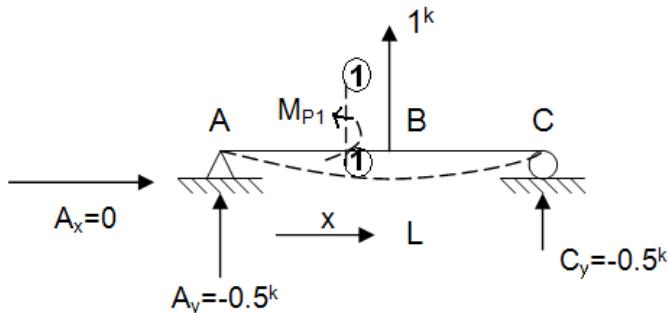
$$W_s + (W_{\text{settle}} = 0) = W_d$$

$$(Q=1^{\text{kft}} \Sigma)(\theta_A) + 0 = \int M_{Q_1} M_{P_1} \frac{dx}{EI}$$

$$\theta_A = \int_0^{12} \left(\frac{x}{12} - 1\right)^2 \frac{dx}{EI} = \dots$$

Example 12

Find the vertical displacement at joint B?



Use left-side of cut section (1)-(1) as FBD.

$$\sum M_{\text{cut}(1)} = 0 = M_{P_1} - (A_y)(x)$$

$$\text{So: } M_{P_1} = A_y x = (-0.5)x \equiv M_{Q_1}$$

Real (P-System) \equiv Virtual (Q- System)

Apply V.W.

$$W_s + W_{\text{settle}} = W_d$$

$$(Q=1^k \uparrow)(\delta_{BV}) + 0 = \int M_{Q_1} M_{P_1} \frac{dx}{EI}$$

$$\delta_{BV} = \int_0^L (-0.5 x)^2 \frac{dx}{EI} = \dots$$